

7. RECAP: GOVERNING EQUATIONS

STRAIN-DISPLACEMENT

$$\underline{\underline{\epsilon}} = \frac{1}{2} (\underline{\nabla} \otimes \underline{u} + \underline{u} \otimes \underline{\nabla}) \quad \underline{\underline{7.1}}$$

$$\text{in 1D: } \epsilon = \frac{\partial u}{\partial x} \quad \underline{\underline{7.2}}$$

CONSTITUTIVE RELATIONS

$$\underline{\underline{\sigma}} = \underline{\underline{C}} \underline{\underline{\epsilon}} \quad \underline{\underline{7.3}}$$

$$\text{in 1D: } \sigma = E \epsilon \quad \underline{\underline{7.4}}$$

EQUATIONS OF MOTION

$$\underline{\nabla} \cdot \underline{\underline{\sigma}} + \underline{f} = \rho \underline{a} \quad \underline{\underline{7.5}}$$

$$\text{in 1D} \quad \frac{\partial \sigma}{\partial x} + f_x = \rho a_x \quad \underline{\underline{7.6}}$$

As an illustration, let's focus on 1D:

$$\frac{\partial \sigma}{\partial x} + f = \rho a \quad \left. \vphantom{\frac{\partial \sigma}{\partial x}} \right\} \underline{\underline{7.4}}$$

$$\frac{\partial}{\partial x} (E \epsilon) + f = \rho a \quad \left. \vphantom{\frac{\partial}{\partial x}} \right\} \underline{\underline{7.2}}$$

$$\frac{\partial}{\partial x} \left(E \frac{\partial u}{\partial x} \right) + f = \rho a$$

$$\boxed{\frac{\partial^2 u}{\partial x^2} \quad \quad \quad \frac{\partial^2 u}{\partial x^2}}$$

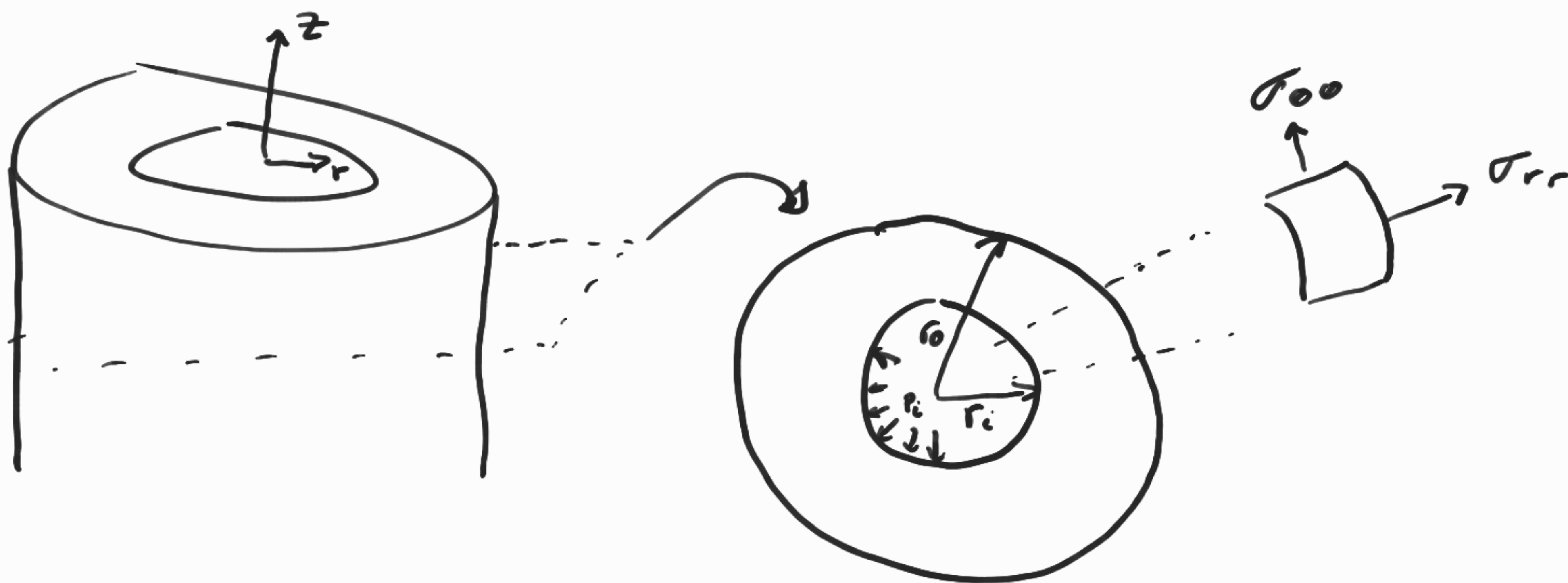
$$\boxed{E \frac{\partial^2 u}{\partial x^2} + f = \rho \frac{\partial^2 u}{\partial t^2}}$$

\Rightarrow SOLID MECHANICS IS DIFFEQ!

8. STRESS ANALYSIS OF AN ARTERY

WE WILL MODEL AN ARTERY AS A THICK-WALLED CYLINDER INFLATED BY AN INTERNAL PRESSURE p_i .

FIND THE DISPLACEMENT DUE TO p_i AND THE STRESS "DISTRIBUTION"



ASSUMPTIONS

1. LINEAR ELASTIC, ISOTROPIC

2. NO EXTERNAL LOADS, NO SHEAR

LET DISPLACEMENT $\underline{u}(x)$ BE DESCRIBED BY

$$\underline{u} = u_r \underline{e}_r + u_\theta \underline{e}_\theta + u_z \underline{e}_z \quad (8.1)$$

BOUNDARY CONDITIONS

$$\text{At } r = r_i, \quad \sigma_{rr} = -p_i, \quad \sigma_{rz} = \sigma_{r\theta} = 0$$

$$\text{At } r = r_o, \quad \sigma_{rr} = \sigma_{rz} = \sigma_{r\theta} = 0$$

8.2

SYMMETRY CONDITION

$$\frac{\partial}{\partial \theta} = 0$$

8.3

More Assumptions

1. $\sigma_{ij} = \sigma_{ij}(r)$, $\epsilon_{ij} = \epsilon_{ij}(r)$, $u_r = u_r(r)$
2. ALL SHEAR STRESS + STRAIN = 0 (r, θ, z = Principal axes)
3. $\sigma_{zz} = 0$ throughout
4. IGNORE GRAVITY.

STRAIN - DISPLACEMENT IN CYLINDRICAL COOR.

$$\epsilon_{rr} = \frac{\partial u_r}{\partial r}$$

$$\epsilon_{\theta\theta} = \frac{u_r}{r}$$

$$\epsilon_{zz} = \frac{\partial u_z}{\partial z}$$

8.4

CONSTITUTIVE EQ'S IN CYL.

$$\sigma_{rr} = \frac{E}{1-\nu^2} (\epsilon_{rr} + \nu \epsilon_{\theta\theta})$$

$$\sigma_{\theta\theta} = \frac{E}{1-\nu^2} (\epsilon_{\theta\theta} + \nu \epsilon_{rr})$$

8.5

EQUILIBRIUM EQ'S

$$\frac{\partial \sigma_{rr}}{\partial r} + \frac{\sigma_{rr} - \sigma_{\theta\theta}}{r} = 0$$

8.6

TO BEGIN, SUBSTITUTE STRAIN-DISPL. 8.4 INTO

CONSTIT. REL. 8.5:

$$\sigma_{rr} = \frac{E}{1-\nu^2} \left(\frac{du_r}{dr} + \nu \frac{u_r}{r} \right)$$

$$\sigma_{\theta\theta} = \frac{E}{1-\nu^2} \left(\frac{u_r}{r} + \nu \frac{\partial u_r}{\partial r} \right)$$

8.7a,b

FINALLY, SUB 8.7 INTO 8.6:

$$\boxed{\frac{d^2 u_r}{dr^2} + \frac{1}{r} \frac{du_r}{dr} - \frac{u_r}{r^2} = 0} \quad \underline{\underline{8.8}}$$

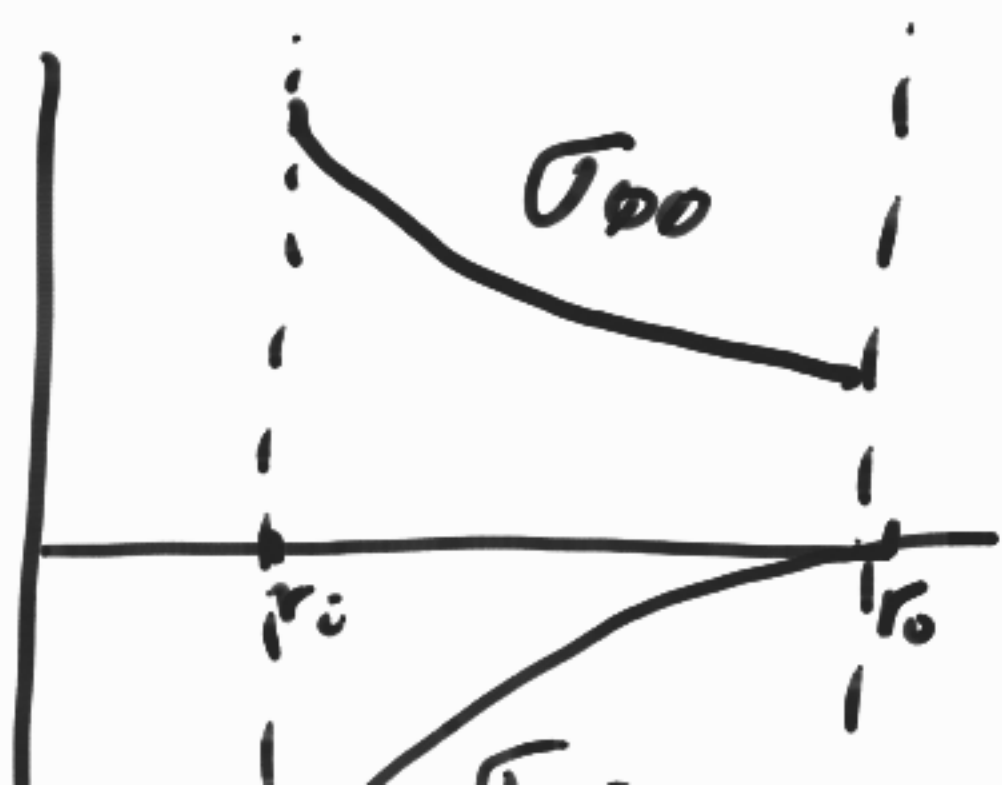
THE GENERAL SOLUTION FOR SUCH A PROBLEM IS

$$u_r(r) = C_1 r + \frac{C_2}{r} \quad (\underline{\underline{8.9}})$$

NOW APPLY B.C. TO SOLVE C_1 AND C_2 , GIVING EXACT SOLUTION

$$\sigma_{rr} = \frac{p_i r_i^2}{r_o^2 - r_i^2} \left(1 - \frac{r_o^2}{r^2} \right)$$

$$\sigma_{\theta\theta} = \frac{p_i r_i^2}{r_o^2 - r_i^2} \left(1 + \frac{r_o^2}{r^2} \right)$$



$$-p_i + \frac{1}{i} \frac{d}{dt} \left(\frac{1}{i} \right)$$

9. Beam bending

BEAM - A geometry in which one dimension is considerably larger than the other two.

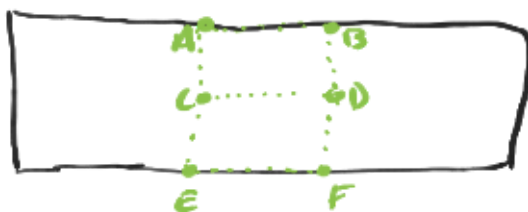
A CANTILEVER BEAM



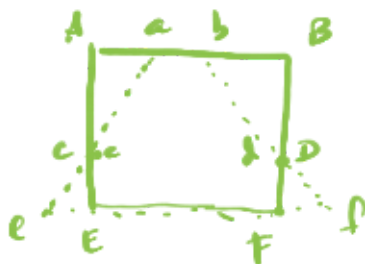
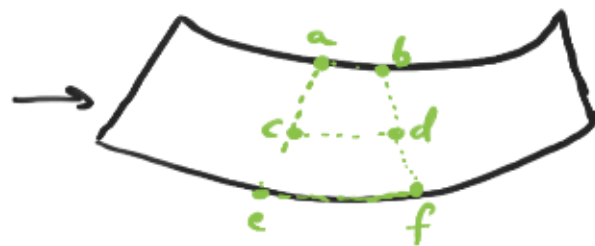
A SIMPLY SUPPORTED BEAM



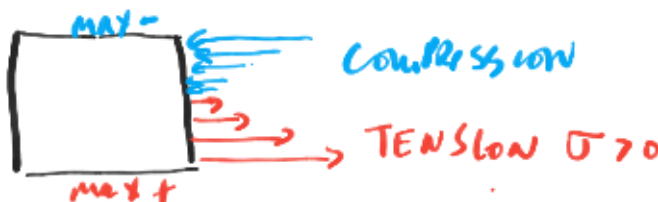
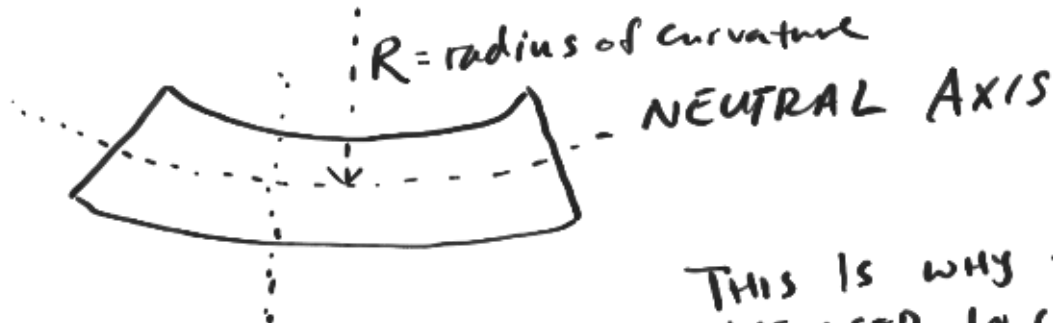
UNDEFORMED



DEFORMED



$\overline{ab} < \overline{AB}$ COMPRESSION
 $\overline{cd} = \overline{CD}$ ZERO STRESS
 $\overline{ef} > \overline{EF}$ TENSION




THIS IS WHY I BEAMS ARE USED IN CONSTRUCTION



NEUTRAL AXIS - LINE THROUGH BEAM LONG AXIS THAT


EXPERIENCES NO STRESS OR STRAIN. FOR SYM. CROSS SECTION AND ISOTROPIC MAT'L WITH NO INITIAL CURVATURE, NEUTRAL AXIS IS THROUGH CENTROID.

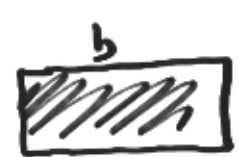
DEFLECTION - DISPLACEMENT OF BEAM \perp to LONG AXIS



$\delta = \text{DEFLECTION}$

SECOND MOMENT OF INERTIA - Geometric Property of 2D surface describing how points in that surface are distributed w.r.t. Some Axis.

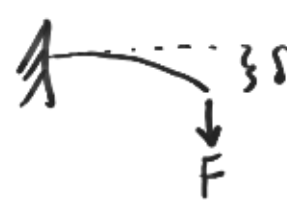

 $I = \frac{\pi r^4}{4}$ 9.1


 $I = \frac{ab^3}{12}$ or $\frac{a^3b}{12}$ 9.2
ab

$M = EI \frac{1}{R}$ 9.3

EI - FLEXURAL RIGIDITY


FOR A CANTILEVER



$F = \frac{3EI}{L^3} \delta$

kb bending stiffness

For 3 POINT BENDING $\delta = \frac{FL^3}{48EI}$



Last modified: Sep 26, 2018